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March 24th, 2020

File #: 17705

City of Coquitlam 3000 Guildford Way Coquitlam, BC V3B 7N2

Attention: Ed Castaneda

Re: Geotechnical Investigation – Robinson memorial Park Cemetery (Phase 1) 621 Robinson Street, Coquitlam, BC

1.0 INTRODUCTION

We understand that The City of Coquitlam is considering the above referenced property for redevelopment. We understand that the Memorial Forest redevelopment will maintain a buffer along the north west, site grading, removing trees elsewhere in the Phase 1, construction of meandering trails through the Green Burial grounds, provision of a new pedestrian connection of the northern half of cemetery, extending the central corridor/road to connect to Robinson street for secondary vehicular access, the addition of 380 traditional burial plots at the north half of the cemetery, grade and site preparation for additional cremated remains plots in the South of cemetery.

This report presents the results of a geotechnical investigation of the soil and groundwater conditions on site and provides preliminary recommendations for the design and construction of the contemplated redevelopment. The report has been prepared exclusively for The City of Coquitlam, for their use and the use of others on their design and construction team.

2.0 SITE DESCRIPTION

The site is located at the southeast corner of the intersection of Robinson Street and Foster Avenue, in Coquitlam, BC. The site is rectangular in shape. The Phase 1 site is bounded by Foster Avenue to the north, The Vancouver Golf Club property to the east, Robinson Street to the west and remaining cemetery lands to the south. At the time of our investigation, the site was predominantly covered with trees.

The location of the site relative to surrounding developments is shown on our site plan (Drawing No. 17705-01), following the text of this report.

3.0 FIELD INVESTIGATION

A geotechnical investigation for this site was completed by GeoPacific Consultants Ltd. February 20th, 2020. The investigation included 9 test pits with depths varying between 2.3 and 3.2 metres below grade. Additionally, in order to estimate in-situ infiltration characteristics of the existing soils, GeoPacific Consultants Ltd. performed Double Ring Infiltration Testing (DRIT) on February 20th, 2020 at the above referenced site. A total of three tests were completed along the north and west side of the development and are shown on our Drawing No. 17705-01 following the text of this letter.

The DRIT was completed using a double ring infiltrometer in accordance with ASTM D3385, modified for the use of electric piezometers for more accurate data collection. The double ring infiltrometer is designed to meet standards for conducting infiltration of water into soils. The DRIT setup consists of two stainless steel rings measuring 300 mm and 600 mm in diameter. The premise of using two rings is to replicate one-dimensional, vertical flow. The water within the annular area will flow vertically and laterally, creating a vertical flow path for the water within the inner ring.

The two rings were driven into the existing soils approximately 100 mm for each soil layer; the smaller ring inside the larger ring, forming a central and annular area. The rings were filled with water and minipiezometers were placed within the rings to record the pressure from the water head. An additional piezometer was placed outside the rings to record atmospheric pressure changes. The piezometers record the pressure in cm of water in 2 second intervals. The readings from the piezometer within the inner ring are normalized by the atmospheric pressure and plotted versus time. The slope of the graph indicates the infiltration rate.

4.0 SUBSURFACE CONDITIONS

4.1 Soil Conditions

The subsurface soil conditions encountered at the locations of our test pits generally consists of up to 0.5 m of sand and gravel fill or forest floor, underlain by dense sand. TP03-20 and TP05-20 show a compact sand and gravel layer beneath the forest floor that extends to depths of up to 1.5 metres below ground surface.

A detailed description of the soils encountered at the site during the investigation is provided in Appendix C. Approximate locations of the test pits are shown on Drawing No. 17705-01.

4.2 Groundwater Conditions

Groundwater was not observed during the geotechnical investigation.

4.3 DRIT and permeability laboratory test results

The depths of the tests were conducted between 1.0 m and 2.5 m below current site grades in dense to very dense sand. The soil profiles and infiltration rates are presented in Table 1 below – detailed results of the infiltration tests are attached in Appendix A. Five laboratory permeability tests were conducted on samples collected from different depths. Table 2 presents the hydraulic conductivity rates and the location and depth of the samples. Detailed results of the permeability tests are attached in Appendix B.

TABLE 1 - SOIL PROFILES AND INFILTRATION RATES

Location	Depth of Test (m)	Infiltration Rate (mm/hr)
DRIT20-01	1.0	315
DRIT20-02	2.5	300
DRIT20-03	2.0	240

TABLE 2 - SOIL PROFILES AND LABORATORY HYDRAULIC CONDUCTIVITY RATES

Location	Depth of Test (m)	Hydraulic Conductivity (mm/hr)
TP20-01	1.2	916.4
TP20-02	2.4	438.5
TP20-03	1.8	760
TP20-07	2.0	1136.7
TP20-08	2.4	900

5.0 DISCUSSION

5.1 General

We understand that some of the areas will be stripped and paved. We also understand that the soil stability and infiltration rates are of interest for future interments.

The native sand to sand and gravel would be well suited to support the future improvements, including walking paths, trails and roads. We confirm, from a geotechnical perspective, that the proposed improvements are feasible provided our recommendations as described in Section 6 of this report are followed.

6.0 DESIGN RECOMMENDATIONS

6.1 Site Preparation

Stripping of organics and/or loose soils will be required prior to placement of the trails, paths and pavement structures. Our minimum stripping depths for foundations, floor slabs, and pavement structures are provided in Table 1 below.

Table 1 - Recommended <u>Minimum</u> Stripping Depths for Foundations, Floor Slabs, & New Pavement Structures

Test Pit	Stripping Depth (metres)	Test Pit	Stripping Depth (metres)
TP20-01	0.6	TP20-06	0.3
TP20-02	0.9	TP20-07	0.5
TP20-03	0.3	TP20-08	0.3
TP20-04	0.3	TP20-09	0.3
TP20-05	0.3		

The stripping depths provided in Table 1 are our <u>minimum</u> stripping depths at each of our test hole locations. It should be appreciated that the depth of stripping can vary across the site. In determining our minimum stripping depths we have measured from the surface downward to the first suitable foundation bearing stratum.

In the context of this report, engineered fill is a well graded gravel that has been systematically placed in controlled lifts, not exceeding 300 mm loose, and compacted to 100% of its Standard Proctor maximum dry density (in accordance with ASTM D698), at a moisture content that is within 2% of optimum for compaction. Engineered fill can be used to support foundation, however it would be preferable to not use fill to support.

6.2 Building Foundations

Based on our investigation, we expect any future buildings to be founded on the native compact to dense sand to sand and gravel. Therefore, any future buildings can be founded on normal spread foundations, including pad and strip footings. Some over-excavation of the existing fills and forest floor could be required.

For subgrade of dense sand prepared as described above, foundations may be designed on the basis of a serviceability limit state (SLS) bearing pressure of 200 kPa and a factored ultimate limit state (ULS) bearing pressure of 300 kPa. For foundations designed as recommended we expect that settlements should be limited to less than 25 mm total and 20 mm over 10 meters differential. For subgrade of compact to dense sand (fill), foundations may be designed on the basis of a serviceability limit state (SLS) bearing pressure of 100 kPa and ultimate limit state (ULS) bearing pressure of 150 kPa.

Irrespective of specified bearing pressures, footings should not be less than 450 mm in width for strip footings and not less than 600 mm in width for square or rectangular footings.

Foundation subgrades must be inspected by the geotechnical engineer prior to construction of foundations.

6.3 Seismic Design of Foundations

The subgrade conditions underlying the site may be classified as <u>Site Class C</u> as defined in Table 4.1.8.4.A of the 2018 British Columbia Building Code. Peak ground accelerations on firm ground for the approximate site location is 0.329 g (National Resource Canada, Site Coordinates: 49.255 degrees North, 122.879 degrees West).

We do not consider the site soils to be prone to liquefaction. Thus, no special ground improvement measures are required to meet the serviceability requirements of the 2019 British Columbia Building Code.

6.4 Grade Supported Slabs

In order to provide suitable support for any grades supported slabs or sidewalks, we recommend that any fill placed under the floor slab should be clear gravel with not more that 5% by weight passing the $75\mu m$ sieve size, with a minimum of 150mm. In addition, this granular fill must be compacted to a standard of 98% of its Standard Proctor maximum dry density within 2% of the optimum moisture content for compaction.

Underslab fill must be reviewed by the geotechnical engineering prior to pouring the slab on grade.

6.5 Excavation stability

As noted above, we understand that part of the site will have excavations for interments up to 2.5 metres deep. During the investigation, vertical cuts up to 3 metres deep remained stable for 2 to 3 hours. Vertical

cuts in the dense sand are expected to remain stable during excavation for a short period of time. If manned entry into the excavations is needed, a professional engineer will be necessary to review the cuts as per Work Safe BC requirements.

6.6 Storm water infiltration and water management

Testing completed in-situ and in the laboratory confirm the range of infiltration values (presented in Section 4.3) for the on-site native sand to sand and gravel. Laboratory permeability testing was done on reconstituted samples, which are at a sufficiently lower relative density, consequently the showed higher rates of permeability. We recommend the Civil Designer to utilize a rate of 200 mm/hr (ultimate) for the design of any infiltration systems.

6.7 Utility Installation

Utility excavations shall be sloped or shored in accordance with the latest Worker's Compensation Board (WCB) Occupational Health & Safety Guidelines. Any excavation in excess of 1.2 metres in height requiring worker-entry must be reviewed by a professional geotechnical engineer.

Light to moderate seepage should be expected during the wet seasons, due to the formation of perched water tables. We expect that inflows may be handled with sumps and sump pumps.

6.8 On-Site Pavements and Roads

Following the recommended site preparation noted in Section 6.1. We envisage that subgrade soils consisting of compact to dense (granular) sand may be re-compacted in-situ with a large vibratory drum roller.

Where existing soils are soft/loose and can not be re-compacted to a minimum of 98% of Maximum Dry Density from a Standard Proctor test (ASTM D698), they must be excavated and replaced with engineered fill.

All base and subbase fills should be compacted to a minimum of 98% of Maximum Dry Density from a Standard Proctor test (ASTM D698).

GeoPacific believes that the minimum pavement structure for the asphalt road, given in Table 2, is sufficient to carry vehicle loads.

Table 2 – Asphalt Structure

Material	Thickness (mm)
Asphaltic Concrete	75
Base course, consisting of 19 mm minus crushed gravel	100
Sub-base course, consisting clean sand and gravel (well graded 100 mm minus)	200

All base and sub-base fills should be compacted to a minimum of 98% of Maximum Dry Density from a Standard Proctor test (ASTM D698), at a moisture content that is within ±2% of its Optimum Moisture Content (OMC).

Compaction tests shall be carried out and in-situ density of the compacted layers shall be measured by GeoPacific.

We envisage that a 300 mm thick layer of compacted 100 mm minus well graded clean sand and gravel can support service vehicles and machinery.

7.0 REVIEWS

The preceding sections make preliminary recommendations for the design and construction of the proposed improvements. We must review final structural design and grades relative to existing grade and update our recommendations as required.

It is the responsibility of the contractors working on-site to inform GeoPacific a minimum of 48 hours in advance that a field review is required. In summary, reviews are required by the geotechnical engineer for the following portions of the work.

Stripping

Review of site stripping.

Foundation

Review of foundation subgrades.

Backfill

Review of backfill materials and compaction.

If you would like further details or clarification please do not hesitate to contact u

For:

GeoPacific Consultants Ltd.

Reviewed by:

M. J. KOKA # 21364

Boris Kolev, M.A.Sc., EIT

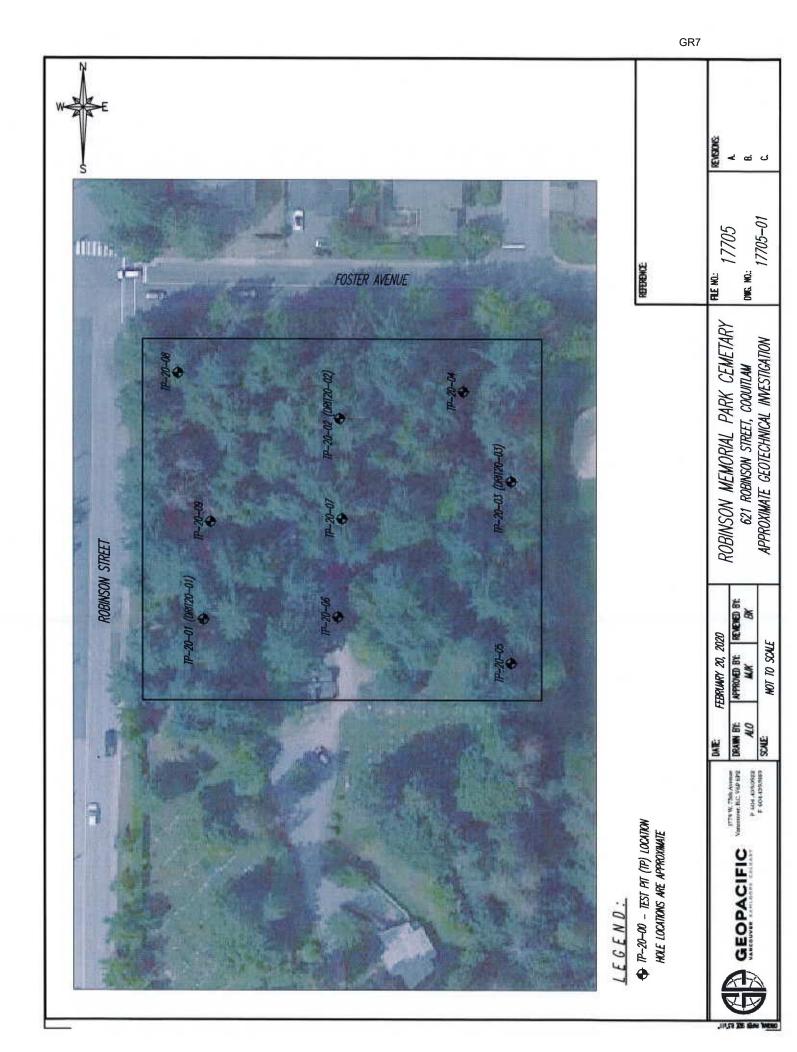
Project Engineer

Matt Kokan, M.A.Sc., P.Eng.

Principal

MAR/2 5 2020

Austin Lockstidt, B.A.Sc., EIT Engineer In Training



Test Pit Log: TP20-01 (DRIT20-01)

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



215 - 1200 West 13rd Avenue, Vancouver, 36, 964 953 Tel: 604-439-9922 Fax:604-439-9109

	INFERRED PROFILE					
	Symbol	Soil Description (m)/Elev (m)		Moisture Content (%)	Groundwater / Well	Romarks
ft m	. No.	Ground Surface	N. W.	1	-	
	3	FOREST FLOOR	0.0			
		SAND Sand, some silt, some organics, loose to compact, moist to wet, light prown	0.2	20.8%		
† †		SAND	0.6			
		Medium grained sand, dense, moist, gray				Infiltration rate of 314.4 mm/hr at 1.0 m.
}						
- 2						
1						
		*** Transmission of		8.7%		
		End of Test Pit	2.5			
7						

Logged: ALO

Method: MINI EXCAVATOR Date: FEBRUARY 19, 2020

Datum: GROUND SURFACE

Figure Number: A.1

Test Pit Log: TP20-02 (DRIT20-02)

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



215 - 1200 West Flad Avenue, Vancouver, 90°, 1009 (CS) Tel: 604-439-0922 Fax:604-439-9189

		INFERRED PROFILE				
Depth	Symbol	Depth (m)/Elev (m)	Moisture Content (%)	Groundwater / Well	Remarks	
oft m	-	Ground Surface				
-		SAND GRAVEL (FILL) Sand and gravel fill, compact, moist, brown	0.0			
	75,000	FOREST FLOOR	0.4	1		
2-		SAND Sand, some silt, trace gravel, moist, brown		11.9%		
3- - 1	<u> </u>	SAND Fine to medium grained sand, dense, moist,	0.9			
4 -		grey.				
6 - - 2						
8						
9-						Infiltration rate of 297.5 mm/hr at 2.5 m
10 - 3			71.75	12.4%		
11-		End of Test Pd	3.5			

Logged ALO

Method: MINI EXCAVATOR
Date FEBRUARY 19, 2020

Datum: GROUND SURFACE

Figure Number: A 7

Test Pit Log: TP20-03 (DRIT20-03)

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



245 - 1200 West 73rd Avenue, Vancouver, BC, VER 6G5 Tel: 604-439-0922 Fax:604-439-9189

		INFERRED PROFILE				
Depth	Symbol	Noith (m)/Elev (m)	Depth (m)/Elev (ரி)	Moisture Content (%)	Groundwater / Well	Remarks
oft m	~_~	Ground Surface FOREST FLOOR	0.0	-		1.4
	171777					
2		SAND GRAVEL Sand and gravel, compact, moist, brown.	0.3	7.0%		
-1		SAND	fø			
4		Medium grained sand, compact, moist, grey				
5-		Becomes dense @ 1 5 m				
5 2						Infiltration rate of 242.7 mm/hr at 2.0 m
8-	H					
9-		End of Test Pis	2.5			
10 - 3						

Logged: ALO

Method: MINI EXCAVATOR

Date: FEBRUARY 19, 2020

Datum: GROUND SURFACE

Figure Number: A.3

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



#15 - 1200 West 73rd Avenue, Vancouver, BC, VBP 6G5 Tel: 604-439-0922 Fax:604-439-9189

		INFERRED PROFILE					
Depth	Symbol	Depth (m)/Elev (m)		Moisture Content (%)	Groundwater / Well	Remarks	
) ft m	72.72	Ground Surface FOREST FLOOR	0.0				
	17.74	POREST FLOOR	0.0				
1		SAND Medium grained sand, trace gravel, dense, moist, tan	0.2				
2							
+		*					
3 - 1							
4		SAND Medium grained sand, some gravel, dense to wery dense, moist, grey.	rr.				
5-							
-							
6-							
1							
- 2		Light seepage @ 2.0 m		10.6%			
7		End of Fest Pit	21				
8							

Logged: ALO

Method MINI EXCAVATOR
Date: FEBRUARY 19, 2020

Datum: GROUND SURFACE

Figure Number: A.4

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



215 - 1200 West 73rd Avenue, Vancouver, BC, V6P 6G5 Tel: 604-439-0922 Fax:604-439-9189

		INFERRED PROFILE				
Depth	Symbol	NOITAINSSAD TIOS Depth (m)/Elev (m)		Moisture Content (%)		Remarks
ft m		Ground Surface				
	22.25	FOREST FLOOR	0.0			
		SAND GRAVEL Sand and gravel, loose to compact, moist, light brown.		8.9%		
-		0.840				
2		SAND Medium grained sand, dense, moist, grey.	1.5	13.2%	2	
		End of Test Pit	2.4			
1 3						

Logged: ALO

Method: MINI EXCAVATOR Date: FEBRUARY 19, 2020 Datum: GROUND SURFACE

Figure Number: A.5

File: 17705

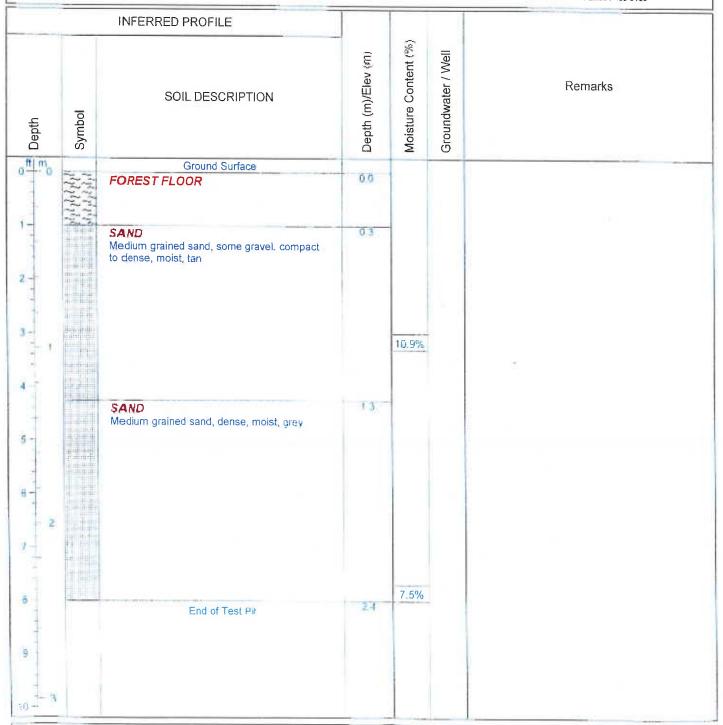
Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



215 - 1200 West 73th Avenue, Vancouver, BC, V6P 6G5 Tel: 604-439-0322 Fax:604-439-9189



Logged: ALO

Method: MINI EXCAVATOR

Date: FEBRUARY 19, 2020

Datum: GROUND SURFACE

Figure Number: A 6

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



215 1890 West 7 Ins Avenue, Vancouver, 50 V6F 6G5 Tel; 604-439-0922 Fax:604-439-9189

	INFERRED PROFILE				
Depth	SOIL DESCRIPTION Oepth (m)/Elev (m)		Moisture Content (%)	Groundwater / Well	Remarks
ft m	Ground Surface				
William I	(distributed)	0.0			
	70 	0.5			
	SAND Medium grained sand, dense, moist, grey.	13			
		23			

Logged; ALO

Method: MINI EXCAVATOR

Date: FEBRUARY 19, 2020

Datum: GROUND SURFACE

Figure Number: A.7

Page. I of 1

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



216 - 1200 West 73rd Avenue, Vancouver, 3C, 1/6P 3G5 Tel: 604-439-0922 Fax:604-439-9189

		INFERRED PROFILE				
Depth	Symbol	Depth (m)/Elev (m) Moisture Content (%)	Moisture Content (%)	Groundwater / Well	Remarks	
oft m	272	Ground Surface	B-0			
4	100 Part 100	FOREST FLOOR	0.0			
2		SAND Medium grained sand, some gravel, compact, moist, brown	0.2	13.0%		
3-		SAND Fine to medium grained sand, dense, moist,	0.8			
4-		grey.				
6 - 2						
8 -						
9		End of Test Pit	2.5			
- 3						

Logged ALO

Method MINI EXCAVATOR

Date: FEBRUARY 19 2020

Datum: GROUND SURFACE

Figure Number: A.8

File: 17705

Project: ROBINSON MEMORIAL PARK CEMETARY

Client: CITY OF COQUITLAM

Site Location: 621 ROBINSON STREET, COQUITLAM



215 - 1200 West 72rd Avenue, Vancouver, EC., VEP 6G5 Tel: 604-439-0922 Fax:604-439-9189

		INFERRED PROFILE				
Depth	Symbol	SOIL DESCRIPTION Depth (m)/Elev (m)		Moisture Content (%)	Groundwater / Well	Remarks
o m o		Ground Surface				
1-		FOREST FLOOR SAND	0.3			
2		Medium grained sand, some gravel, compact, moist, brown.	3.3		-	
3-				12.2%		
-			1			
5		SAND Fine to medium grained sand, dense, moist, grey	12			
6						
7-2		End of Test Pit	7.1	7.5%		
8-		Elid of Test Fix				

Logged ALO

Method: MINI EXCAVATOR

Date FEBRUARY 19, 2020

Datum: GROUND SURFACE

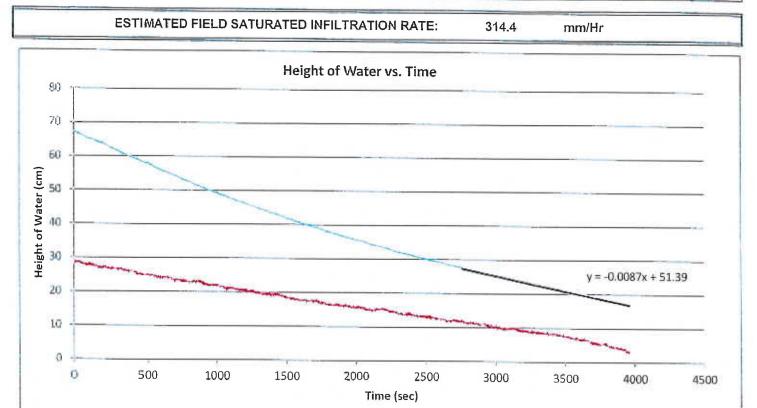
Figure Number: A.9



DOUBLE RING INFILTRATION TEST (ASTM D3385)

Vancouver Lab 1779 WeSt 75th Avenue Vancouver, B.C V6P 6P2

CLIENT:	CITY OF COQUITLAM	PROJECT#:	17705
PROJECT NAME:	ROBINSON MEMORIAL PARK CEMETARY	DATE:	20-Feb-2020 9:00
PROJECT LOCATION:	621 ROBSINSON STREET, COQUITLAM	TEST NO.:	DRIT20-01



		TEST INFOR	RMATION		
LIQUID USED:	WATER	AREA INNER CYLINDER:	72965.63 mm ²	DEPTH OF TEST BELOW	
LIQUID pH:	7.1	ANNULAR AREA:	209777.71 mm ²	CURRENT SITE GRADE (m):	1.0
SOIL DESCRIPTION:	SAND	CYLINDER ACCURACY:	± 0.5 mm	INFILTRATION RATE	0.7775.00
SOIL TYPE:	NATIVE	LIQUID DEPTH INNER:	67.4 cm	(cm/sec);	8.732E-03
WEATHER ON SITE:	CLEAR	LIQUID DEPTH OUTER:	29.2 cm	DEPTH TO WATER TABLE:	UNKNOWN

Comments:

Per: Austin Lockstidt, B.A.Sc., EIT

Enginner In Training

Reviewed by: Jakub Szary, B.Sc., AScT

FEB 2 4 2020

Lab Manager

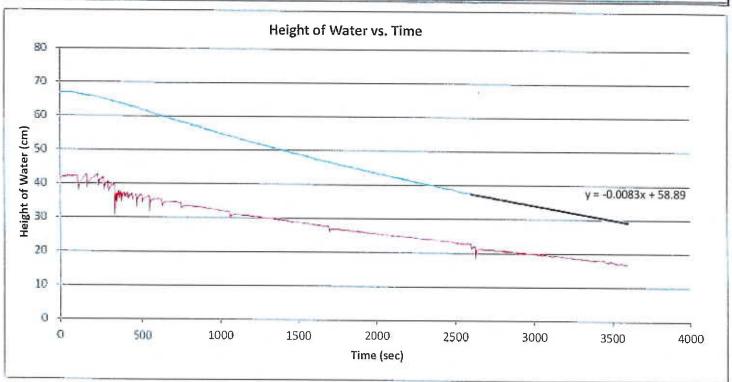


DOUBLE RING INFILTRATION TEST (ASTM D3385)

Vancouver Lab GR18 1779 West 75th Avenue Vancouver, 8.C V6P 6P2

CLIENT:	CITY OF COQUITLAM	PROJECT #:	17705
PROJECT NAME:	ROBINSON MEMORIAL PARK CEMETARY	DATE:	20-Feb-2020 10:00
PROJECT LOCATION:	621 ROBSINSON STREET, COQUITLAM	TEST NO.:	DRIT20-02





		TEST INFOR	MATION		
LIQUID USED:	WATER	AREA INNER CYLINDER:	72965.63 mm ²	DEPTH OF TEST BELOW	
LIQUID pH:	7.1	ANNULAR AREA:	209777.71 mm ²	CURRENT SITE GRADE (m):	2.5
SOIL DESCRIPTION:	SAND	CYLINDER ACCURACY:	± 0.5 mm	INFILTRATION RATE	
SOIL TYPE:	NATIVE	LIQUID DEPTH INNER:	67.1 cm	(cm/sec):	8.263E-03
WEATHER ON SITE:	CLEAR	LIQUID DEPTH OUTER:	42.8 cm	DEPTH TO WATER TABLE:	UNKNOWN

Comments:

Per: Austin Lockstidt, B.A.Sc., EIT

Enginner In Training

Reviewed by: Jakub Szary, B.Sc., AScT

FEB 2-4 2020

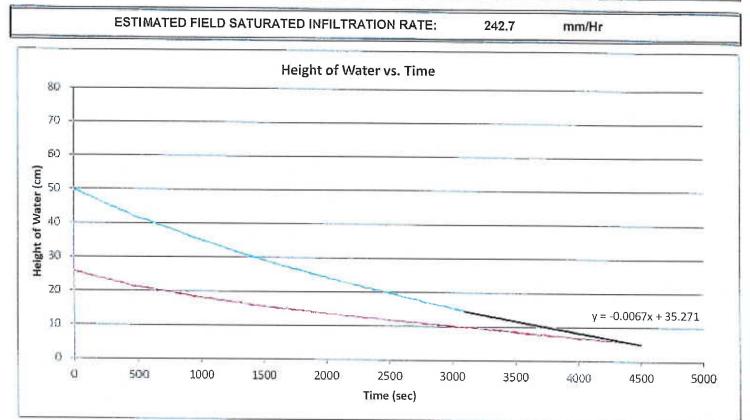
Lab Manager



DOUBLE RING INFILTRATION TEST (ASTM D3385)

Vancouver Lab GR19 1779 West 75th Avenue Vancouver, B.C V6P 6P2

CLIENT:	CITY OF COQUITLAM	PROJECT #:	17705
PROJECT NAME:	ROBINSON MEMORIAL PARK CEMETARY	DATE:	20-Feb-2020 11:00
PROJECT LOCATION:	621 ROBSINSON STREET, COQUITLAM	TEST NO.:	DRIT20-03



		TEST INFOR	RMATION		
LIQUID USED:	WATER	AREA INNER CYLINDER:	72965.63 mm ²	DEPTH OF TEST BELOW	
LIQUID pH:	2.1	ANNULAR AREA:	209777.71 mm ²	CURRENT SITE GRADE (m):	2.0
SOIL DESCRIPTION:	SAMO	CYLINDER ACCURACY:	± 0.5 mm	INFILTRATION RATE	
SOIL TYPE:	NATIVE	LIQUID DEPTH INNER:	49.9 cm	(cm/sec):	6.740E-03
WEATHER ON SITE:	CLEAR	LIQUID DEPTH OUTER:	25.9 cm	DEPTH TO WATER TABLE:	UNKNOWN

Comments:

Per: Austin Luckstell, 8 A.Br. EIT

Enginner In Training

Reviewed by: Jakub Szary, B.Sc., AScT

FEB 2 4 2020

Lab Manager



17705

19-Fen-20

21-Feb-20

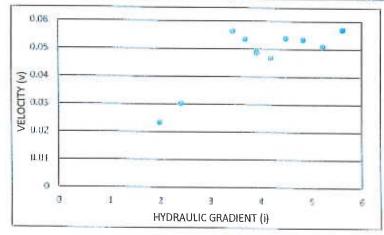
CONSTANT HEAD PERMEABILITY TEST (ASTM D2434)

CLIENT:	CITY OF COQUITLAM	IOF #
PROJECT:	ROBINSON MEMORIAL PARK CEMETARY (PHASE 1)	Ascewed
LOCATION:	621 ROBINSON STREET, COQUITLAM	TESTEO

	SAMPLE DETAIL	.S	
SOIL DESCRIPTION:	SAND	SCREEN SIZE;	19mni
SAMPLE ID:	TP20-02 @ 2.4m	OVERSIZE:	13%

	EQUATIONS & T	EST PARAMETERS (cm)	
HYDRAULIC CONDUCTIVITY:	$h = \frac{Q \times L}{A \times t \times h}$	MOLD AREA (A):	45 4
HYDRAULIC GRADIENT:	$i = \frac{h}{L}$	MANOMETER SPACING (L):	7.6
VELOCITY:	$\nu = \frac{Q}{A \times t}$	DRY DENSITY;	1618 kg/m³

			TE	ST RESUL	TS					
TRIAL#	1	2	3	4	5	6	7.	8	9	10
MANOMETER HEIGHT (h):	15.1	18.3	26.2	28.1	29.8	31.9	34.1	36.7	39.7	42.7
FLOW (Q):	909.9	901.3	682.6	756.5	892,3	971.4	907.2	974.4	1049.3	1077.3
TIME (t):	856	657	266	311	404	457	372	403	452	416
HYDRAULIC GRADIENT (i):	1.99	2.41	3.45	3.70	3.92	4.20	4.49	4.83	5.22	5.62
VELOCITY (v):	2.34E-02	3.02E-02	5.66E-02	5.36E-02	4.87E-02	4.69E-02	5.38E-02	11 10000001		
HYDRAULIC CONDUCTIVITY:	1.18E-02	1.26E-02	1.64E-02	1.45E-02	1.24E-02	1.12E-02	1.20E-02	1,10E-02		1.02E-02



AVG HYDRAULIC CONDUCTIVITY

438.5 mm/hr

COMMENTS:

DISTRIBUTION:

Boris Kovlev, GeoPacific Austin Lockstidt, GeoPacific

Per: Cindy Marinovic, B.Sc.

Reviewed By: Jakub Szary, B.Sc., AScT

tab Manager MAR 1 9 2020

Lab Technician



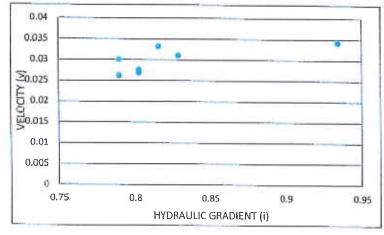
CONSTANT HEAD PERMEABILITY TEST (ASTM D2434)

CLIENT:	CITY OF COQUITLAM	100 #	17705
PROJECT:	ROBINSON MEMORIAL PARK CEMETARY (PHASE 1)	RECEIVE	D: 19-Feb-(3)
LOCATION:	621 ROBINSON STREET, COQUITLAM	TESTED	1 2-Mar-20

	SAMPLE DETAIL	S	
SOIL DESCRIPTION:	SAND	SCREEN SIZE:	19mn
SAMPLE ID:	TP20-01 @ 1.2m	OVERSIZE:	0%

	EQUATIONS & T	EST PARAMETERS (cm)	
HYDRAULIC CONDUCTIVITY:	$k = \frac{Q \times \mathcal{E}}{A \times t \times h}$	MOLD AREA (A):	45.4
HYDRAULIC GRADIENT:	$l = \frac{h}{L}$	MANOMETER SPACING (L):	7.6
VELOCITY:	$v = \frac{Q}{A \times t}$	DRY DENSITY:	1368 kg/m³

TEST RESULTS										
TRIAL #:	1	2	3	4	5	6	7	8	9	10
MANOMETER HEIGHT (h):	6.0	6.1	6.1	6.0	6.3	6.2	7.1			1/4
FLOW (Q):	678.4	694.3	709.2	696.3	702.3	691.4	710,2			_
TIME (t):	569	568	564	509	497	458	458			-
HYDRAULIC GRADIENT (i):	0.79	0.80	0.80	0.79	0.83	0.82	0.93			
VELOÇITY (v):	2.63E-02	2.69E-02	2.77E-02	3.02E-02	3.12E-02	3.33E-02	3.42E-02			_
HYDRAULIC CONDUCTIVITY:	3.33E-02			3.82E-02				_		-



AVG HYDRAULIC CONDUCTIVITY
916.4 mm/hr

COMMENTS

DISTRIBUTION

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Per: Cindy Marinovic, B.Sc.

Reviewed By: Jakub Szary, B.Sc., AScT

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Lab Manager MAR 9 9 2020



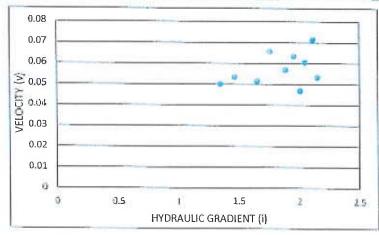
CONSTANT HEAD PERMEABILITY TEST (ASTM D2434)

CLIENT:	CITY OF COQUITLAM	308.4	17795
PROJECT:	ROBINSON MEMORIAL PARK CEMETARY (PHASE 1)	RECEIVED	19-Fab-20
LOCATION:	621 ROBINSON STREET, COQUITLAM	TESTED	27-Feb-26

LOCATION: 621 ROBI	NSON STREET, COQUITLAM		resteu :	27-Feb-20
	SAMPLE DETAILS			
SOIL DESCRIPTION:	SAND, SOME GRAVEL	SCREEN SIZE:	9.5m	m
SAMPLE ID:	TP20-07 @ 2m	OVERSIZE:	7%	

	EQUATIONS & T	EST PARAMETERS (cm)	
HYDRAULIC CONDUCTIVITY:	YDRAULIC CONDUCTIVITY: $k = \frac{Q \times L}{A \times t \times h}$		45.4
HYDRAULIC GRADIENT:	$t = \frac{h}{L}$	MANOMETER SPACING (L):	7.6
VELOCITY:	$v = \frac{Q}{A \times t}$	DRY DENSITY:	1401 kg/m²

			TE	ST RESUL	TS					
TRIAL#:	1 1	2	3	4	- 5	6	7	8	9	10
MANOMETER HEIGHT (h):	10.3	11.2	12.6	13.4	14.4	14.9	15.6	16.4	15.3	16.1
FLOW (Q):	762.3	765.9	817.7	710.2	852.8	803.4	931.5	1204.6	1249,2	737.8
TIME (t):	335	315	349	238	330	278	339	497	588	227
HYDRAULIC GRADIENT (I):	1.36	1.47	1.66	1.76	1.89	1.96	2.05	2.16	2.01	2.12
VELOCITY (v):	5.02E-02	5.36E-02	5.16E-02	6.58E-02	5.70E-02	6.37E-02	6.06E-02	5.34E-02		
HYDRAULIC CONDUCTIVITY:	3.70E-02	3.64E-02	3.12E-02	3.73E-02	3.01E-02	3.25E-02	2.95E-02	2.48E-02	2.33E-02	3.38E-02



AVG HYDRAULIC CONDUCTIVITY

1136.7 mm/hr

COMMENTS:

DISTRIBUTION:

Boris Kovlev, GeoPacific

Austin Lockstidt, GeoPacific

Per: Cindy Marinovic, B.Sc.

Reviewed By: Jakub Szary, B.Sc., AScT

MAR 1 9 2020

Lab Manager

Lab Technician



FALLING HEAD PERMEABILITY TEST

CLIENT:	CITY OF COQUITLAM	JOB#:	17705
PROJECT:	ROBINSON MEMORIAL PARK CEMETARY (PHASE 1)	RECEIVED:	19-Feb-20
LOCATION:	621 ROBINSON STREET, COQUITLAM	TESTED:	21-Feb-20

	SAMPLE DETAILS		
SOIL DESCRIPTION:	FINE SAND, SOME SILT	SCREEN SIZE:	19 mm
SAMPLE ID;	TP20-03 @ 1.8m	OVERSIZE:	0.3%

EQUATIONS & TEST PARAMETERS (cm)						
HYDRAULIC CONDUCTIVITY:	/ITY: $k = \frac{aL}{At} \ln \frac{h_0}{h_1}$ MOLD WEIGHT		10301			
MOLD AREA (A)	181.5	MOLD & SOIL WEIGHT (g):	14387.9			
RESERVOIR AREA (a):	165.1	COMPACTION MOISTURE:	15.3%			
MOLD LENGTH (L):	12.25	DRY DENSITY (kg/m³):	1594			

			TES	T RESULT	Ş					
TRIAL #:	1	2	3	4	9	6	7	8	9	10
INITIAL HEIGHT (ho):	48.6	43.00	39.0	34.3	28.6	24.9	20.9	18.0	14.4	11.8
FINAL HEIGHT (h1):	43.20	39.1	34.5	31.6	25.0	21.0	18.1	14.5	11.9	9,6
TIME (t):	941	710	916	613	716	886	691	935	734	718
HYDRAULIC CONDUCTIVITY:	1.39E-03	1.49E-03	1.49E-03	1.49E-03	2.09E-03	2.14E-03	2.32E-03	2.58E-03	2.89E-03	3.20E-03

AVG HYDRAULIC CONDUCTIVITY 7.60E+01 mm/hr

COMMENTS:

DISTRIBUTION:

Boris Kovlev, GeoPacific

Austin Lockstidt, GeoPacific

Per: Cindy Marinovic, B.Sc.

Reviewed By: Jakub Szary, B.Sc., AScT

Lab Mariager MAR 0 9 2020

Lab Technician



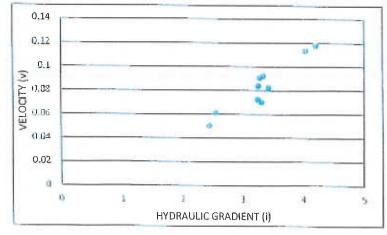
CONSTANT HEAD PERMEABILITY TEST (ASTM D2434)

CLIENT:	CITY OF COQUITLAM	.108 #	17705
PROJECT:	ROBINSON MEMORIAL PARK CEMETARY (PHASE 1)	RECEIVED	19-Feb-20
LOCATION:	621 ROBINSON STREET, COQUITLAM	TESTED	4-Mar-20

	SAMPLE DETAILS		
SOIL DESCRIPTION:	SAND, SOME GRAVEL, TRACE SILT	SCREEN SIZE;	19mm
SAMPLE ID:	TP20-08 @ 2.4m	OVERSIZE:	0.6%

	EQUATIONS & T	EST PARAMETERS (cm)	
HYDRAULIC CONDUCTIVITY:	$k = \frac{\overline{Q} \times L}{A \times t \times h}$	MOLD AREA (A):	45.4
HYDRAULIC GRADIENT:	$i = \frac{h}{L}$	MANOMETER SPACING (L):	76
VELOCITY:	$v = \frac{Q}{A \times t}$	DRY DENSITY:	1593 kg/m³

			TE	ST RESUL	TS			1000		
TRIAL #;	1	2	-3	4	5	ő	7	6	9	10
MANOMETER HEIGHT (h):	30.7	32.0	24.7	25.2	24.8	25.4	25.0	26.1	19.4	18.6
FLOW (Q):	753.8	739.3	939.6	992.8	813.6	751.4	772.5	815.9	805.1	956.4
TIME (t):	146	138	285	311	213	179	187	217	287	417
HYDRAULIC GRADIENT (i):	4.04	4.21	3.25	3,32	3.26	3.34	3.29	3.43	2.55	2.45
VELOCITY (v):	1.14E-01	1.18E-01	7.27E-02	7.04E-02	8.42E-02	9.25E-02		8.29E-02		5.06E-02
HYDRAULIC CONDUCTIVITY:	2.82E-02	2.80E-02	2.24E-02	2.12E-02	2.58E-02	2.77E-02	2.77E-02			



AVG HYDRAULIC CONDUCTIVITY
900.0 mm/hr

COMMENTS:

DISTRIBUTION:

Boris Kovlev, GeoPacific

Austin Lockstidt, GeoPacific

Per: Cindy Marinovic, B.Sc.

Reviewed By: Jakub Szary, B.Sc., AScT

Lab Technician

Lab Manage,MAR 1 9 2020

Appendix CEnvironmental Soils Report



November 10, 2020

Reference: 10682/V4917

Urban Systems Ltd. 550-1090 Homer Street Vancouver BC V6B 2W9

Attn: Michael Vorhies

Re: Robinson Memorial Park Cemetery Expansion Project Soil Sampling with Increased Scope

Dear Michael,

Triton Environmental Consultants Ltd. (Triton) was retained by Urban Systems to sample and review results for soil samples at Robinson memorial Park Cemetery in Coquitlam, BC (the Site). Sample results are compared to the BC Contaminated Sites Regulation (CSR) Schedule 3.1 – Soil Standards – Urban Park (PL), Residential Low Density (RLLD), Residential High Density (RLHD) and Industrial (IL). Samples are collected following appropriate field sampling and sample handling procedures (BC Field Sampling Manual 2013). A summary of sample information and results is provided below (Table 1), with the third-party analytical laboratory (AGAT Laboratories) Certificate of Analysis (20V624985) available in Appendix C and a comparison of sample results to CSR PL, CSR RLLD and CSR RLHD, and CSR IL Standards in Appendix D.

Table 1. CSR Schedule 3.1 Soil Standard Exceedances by Land Use

Sample Site	Depth (m)	< Commercial (CL) ¹	> Industrial (IL) ²
Wooded Area	0.5 – 1.0	-	-
(NW1)	2.0 – 2.5	-	-
Wooded Area	0.5 – 1.0	-	-
(NW2)	2.0 – 2.5	-	-
Wooded Area	0.5 – 1.0	-	-
(NW3)	2.0 – 2.5	-	-
Wooded Area	0.5 – 1.0	-	-
(NE)	2.0 – 2.5	-	-
	0 – 0.5	-	-
	0.5 – 1.0	Chloride	-
South Road (S1)	1.0 – 1.5	-	-
	1.5 – 2.0	-	-
	2.0 - 2.5	-	-
	0 – 0.5	-	Chloride
South Road (S2)	0.5 – 1.0	-	Chloride, Selenium
	1.0 – 1.5	Chloride	-

	1.5 – 2.0	Chloride	-
	2.0 – 2.5	-	-
	0 – 0.5	Chloride	-
South Road (S3)	0.5 – 1.0	-	Chloride
	1.0 – 1.5	-	Chloride
	1.5 - 2.0	Chloride	-
	2.0 - 2.5	-	Chloride
	0 – 0.5	-	Chloride
	0.5 – 1.0	Chloride	-
South Road (S4)	1.0 – 1.5	Chloride	-
	1.5 - 2.0	-	-
	2.0 – 2.5	-	-

¹ – Exceeds all land uses except Commercial and Industrial land uses

² – Exceeds all land uses

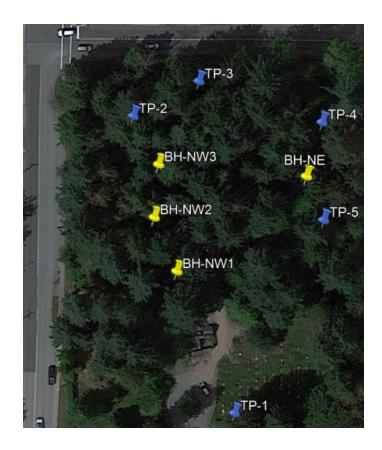


Figure 1. Soil sampling locations for North Wooded Area (Google 2020)



Figure 2. Soil sampling locations for South Cemetery Road (Google 2020)

Table 2. Soil sample information and results

Sample ID	Date	Time	Sampler	Parameter	Within CSR PL?	Within CSR RL _{LD} ?	Within CSR RL _{HD} ?	Within CSR IL?
BH-NW1-U (0.5-1.0)	13-Jul-20	10:30	AO	LEPH/HEPH & PAH	V	Ø	Ø	V
BH-NW1-U (0.5-1.0)	13-Jul-20	10:30	AO	CSR Metals ¹	Ø	V	\square	Ø
BH-NW1-L (2.0-2.5)	13-Jul-20	10:30	AO	LEPH/HEPH & PAH	Ø	\square	Ø	Ø
BH-NW1-L (2.0-2.5)	13-Jul-20	10:30	AO	VPH/ BTEX / VOCs	Ø	Ø	Ø	V
BH-NW1-L (2.0-2.5)	13-Jul-20	10:30	AO	CSR Metals ¹	Ø	Ø	Ø	V
BH-NW2-U (0.5-1.0)	13-Jul-20	9:40	AO	LEPH/HEPH & PAH	Ø		Ø	Ø
BH-NW2-U (0.5-1.0)	13-Jul-20	9:40	AO	CSR Metals ¹	Ø	V	Ø	V
BH-NW2-L (2.0-2.5)	13-Jul-20	9:40	AO	LEPH/HEPH & PAH	Ø	Ø	Ø	V
BH-NW2-L (2.0-2.5)	13-Jul-20	9:40	AO	VPH/ BTEX / VOCs	Ø	Ø	Ø	V
BH-NW2-L (2.0-2.5)	13-Jul-20	9:40	AO	CSR Metals ¹	Ø		Ø	Ø
BH-NW3 (0-0.5)	13-Jul-20	9:00	AO	LEPH/HEPH & PAH	Ø		Ø	Ø
BH-NW3 (0-0.5)	13-Jul-20	9:00	AO	CSR Metals ¹	Ø	V	Ø	V
BH-NW3-L (2.0-2.5)	13-Jul-20	9:00	AO	LEPH/HEPH & PAH	Ø	Ø		V
BH-NW3-L (2.0-2.5)	13-Jul-20	9:00	AO	VPH/ BTEX / VOCs	Ø	Ø	Ø	Ø
BH-NW3-L (2.0-2.5)	13-Jul-20	9:00	AO	CSR Metals ¹	Ø	V	V	V
BH-NE-U (0.5-1.0)	13-Jul-20	11:00	AO	LEPH/HEPH & PAH	Ø	V	Ø	Ø
BH-NE-U (0.5-1.0)	13-Jul-20	11:00	AO	CSR Metals ¹	Ø	V	Ø	V
BH-NE-L (2.0-2.5)	13-Jul-20	11:00	AO	LEPH/HEPH & PAH	Ø	Ø	Ø	\square
BH-NE-L (2.0-2.5)	13-Jul-20	11:00	AO	VPH/ BTEX / VOCs	Ø	V	V	V
BH-NE-L (2.0-2.5)	13-Jul-20	11:00	AO	CSR Metals ¹	Ø	V	Ø	V
BH-S1 (0-0.5)	13-Jul-20	13:00	AO	CSR Metals ¹	Ø	Ø	Ø	V
BH-S1-U (0.5-1.0)	13-Jul-20	13:00	AO	LEPH/HEPH & PAH	Ø	Ø	Ø	V
BH-S1-U (0.5-1.0)	13-Jul-20	13:00	AO	CSR Metals ¹	X 3	X 3	X 3	Ø
BH-S1 (1.0-1.5)	13-Jul-20	13:00	АО	CSR Metals ¹	Ø	Ø	Ø	V

Sample ID	Date	Time	Sampler	Parameter	Within CSR PL?	Within CSR RL _{LD} ?	Within CSR RLHD?	Within CSR IL?
BH-S1 (1.5-2.0)	13-Jul-20	13:00	AO	CSR Metals ¹	V	Ø	Ø	$\overline{\checkmark}$
BH-S1-L (2.0-2.5)	13-Jul-20	13:00	AO	LEPH/HEPH & PAH	Ø	\square	Ø	
BH-S1-L (2.0-2.5)	13-Jul-20	13:00	AO	VPH/ BTEX / VOCs	Ø	\square	Ø	$\overline{\checkmark}$
BH-S1-L (2.0-2.5)	13-Jul-20	13:00	AO	CSR Metals ¹	Ø	\square	Ø	$\overline{\checkmark}$
BH-S2 (0-0.5)	13-Jul-20	13:00	AO	CSR Metals ¹	× 3	× 3	× 3	× 3
BH-S2-U (0.5-1.0)	13-Jul-20	12:30	AO	LEPH/HEPH & PAH		Ø	Ø	$\overline{\checkmark}$
BH-S2-U (0.5-1.0)	13-Jul-20	12:30	AO	CSR Metals ¹	x 2,3	x 2,3	x 2,3	x 2,3
BH-S2 (1.0-1.5)	13-Jul-20	13:00	AO	CSR Metals ¹	× 3	× 3	× 3	$\overline{\checkmark}$
BH-S2 (1.5-2.0)	13-Jul-20	13:00	AO	CSR Metals ¹	× 3	× 3	× 3	$\overline{\checkmark}$
BH-S2-L (2.0-2.5)	13-Jul-20	12:30	AO	LEPH/HEPH & PAH	Ø	Ø	Ø	$\overline{\checkmark}$
BH-S2-L (2.0-2.5)	13-Jul-20	12:30	AO	VPH/ BTEX / VOCs	Ø	\square	Ø	$\overline{\checkmark}$
BH-S2-L (2.0-2.5)	13-Jul-20	12:30	AO	CSR Metals ¹	Ø	Ø	Ø	$\overline{\checkmark}$
BH-S3 (0-0.5)	13-Jul-20	13:00	AO	CSR Metals ¹	× 3	× 3	× 3	$\overline{\checkmark}$
BH-S3-U (0.5-1.0)	13-Jul-20	12:00	AO	LEPH/HEPH & PAH	V	Ø	Ø	$\overline{\mathbf{V}}$
BH-S3-U (0.5-1.0)	13-Jul-20	12:00	AO	CSR Metals ¹	× 3	× 3	× 3	x 3
BH-S3 (1.0-1.5)	13-Jul-20	13:00	AO	CSR Metals ¹	× 3	× 3	× 3	× 3
BH-S3 (1.5-2.0)	13-Jul-20	13:00	AO	CSR Metals ¹	× 3	× 3	× 3	V
BH-S3-L (2.0-2.5)	13-Jul-20	12:00	AO	LEPH/HEPH & PAH		Ø	Ø	V
BH-S3-L (2.0-2.5)	13-Jul-20	12:00	AO	VPH/ BTEX / VOCs	Ø	Ø	Ø	V
BH-S3-L (2.0-2.5)	13-Jul-20	12:00	AO	CSR Metals ¹	× 3	× 3	× 3	x 3
BH-S4 (0-0.5)	13-Jul-20	13:00	АО	CSR Metals ¹	× 3	× 3	× 3	x 3
BH-S4-U (0.5-1.0)	13-Jul-20	11:30	AO	LEPH/HEPH & PAH	Ø	V	Ø	
BH-S4-U (0.5-1.0)	13-Jul-20	11:30	AO	CSR Metals ¹	× 3	× 3	× 3	
BH-S4 (1.0-1.5)	13-Jul-20	13:00	AO	CSR Metals ¹	× 3	X 3	× 3	V

Sample ID	Date	Time	Sampler	Parameter	Within CSR PL?	Within CSR RL _{LD} ?	Within CSR RL _{HD} ?	Within CSR IL?
BH-S4 (1.5-2.0)	13-Jul-20	13:00	AO	CSR Metals ¹	Ø	V	7	V
BH-S4-L (2.0-2.5)	13-Jul-20	11:30	AO	LEPH/HEPH & PAH	Ø	☑	Ø	V
BH-S4-L (2.0-2.5)	13-Jul-20	11:30	AO	VPH/ BTEX / VOCs	Ø		Ø	Ø
BH-S4-L (2.0-2.5)	13-Jul-20	11:30	AO	CSR Metals ¹	Ø		Ø	Ø
TP-1	13-Jul-20	11:30	AO	LEPH/HEPH & PAH	\square	\square	\square	\square
TP-1	13-Jul-20	11:30	AO	CSR Metals ¹			\square	
TP-2	13-Jul-20	11:30	AO	LEPH/HEPH & PAH		V	\square	V
TP-2	13-Jul-20	11:30	AO	CSR Metals ¹	\square	Ø	\square	
TP-3	13-Jul-20	11:30	AO	LEPH/HEPH & PAH			\square	
TP-3	13-Jul-20	11:30	AO	CSR Metals ¹	\square	Ø	Ø	\square
TP-4	13-Jul-20	11:30	AO	LEPH/HEPH & PAH	\square	Ø	Ø	
TP-4	13-Jul-20	11:30	AO	CSR Metals ¹	Ø	Ø	Ø	Ø
TP-5	13-Jul-20	11:30	AO	LEPH/HEPH & PAH	\square	Ø	Ø	Ø
TP-5	13-Jul-20	11:30	AO	CSR Metals ¹	\square	V	Ø	V

¹ Na, CI analysed by saturated paste method

² Selenium exceeded guidelines

³ Chloride exceeded guidelines



Based on the soil results, only the south road soils require disposal at an appropriate receiving site. Figure 3 below indicates how the south road could be delineated for disposal purposes.

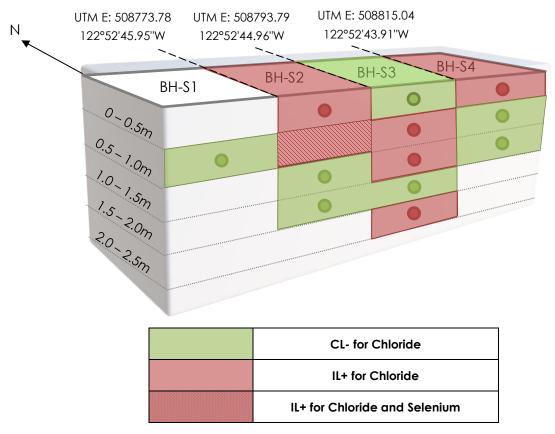


Figure 3. Delineation of contaminated soil onsite at South Road

BH-S1 (From Robinson Street to UTM Easting 508773.78)

- 0.5–1.0m contaminated with levels of chloride less than Commercial land use guidelines

BH-S2 (From UTM Easting 508773.78 to UTM Easting 508793.79)

- 0–0.5m contaminated with levels of chloride in exceedance of Industrial land use guidelines
- 0.5–1.0m contaminated with levels of chloride and selenium in exceedance of Industrial land use guidelines
- 1.0–2.0m contaminated with levels of chloride less than Commercial land use guidelines

BH-S3 (From UTM Easting 508793.79 to UTM Easting 508815.04)

- 0-0.5m contaminated with levels of chloride less than Commercial land use guidelines
- 0.5–1.5m contaminated with levels of chloride and selenium in exceedance of Industrial land use guidelines
- 1.5–2.0m contaminated with levels of chloride less than Commercial land use guidelines
- 2.0–2.5m contaminated with levels of chloride in exceedance of Industrial land use guidelines

BH-S4 (From UTM Easting 508815.04 to internal road intersection)

- 0–0.5m contaminated with levels of chloride in exceedance of Industrial land use guidelines
- 0.5–1.5m contaminated with levels of chloride less than Commercial land use guidelines

The south road should be delineated horizontally with the UTM coordinates listed above. Soil can be delineated vertically as listed above at each individual depth. All soil with results less than Commercial (CL) guidelines can disposed of at receiving sites with Commercial and Industrial land uses. Any soil with results greater than Industrial (IL) guidelines must be disposed of at an appropriate facility permitted to accept the material.

Please contact us if you have any questions or concerns about the report.

Regards,

Triton Environmental Consultants Ltd.

Adrian On, B.Sc., B.Tech., BC-CESCL

Environmental Professional

Miranda Lewis, P.Ag., M.Sc., BC-CESCL

Mpander Lin

Environmental Professional

Attachments: Appendix A – Urban Systems Site Drawing

Appendix B – Borehole Logs

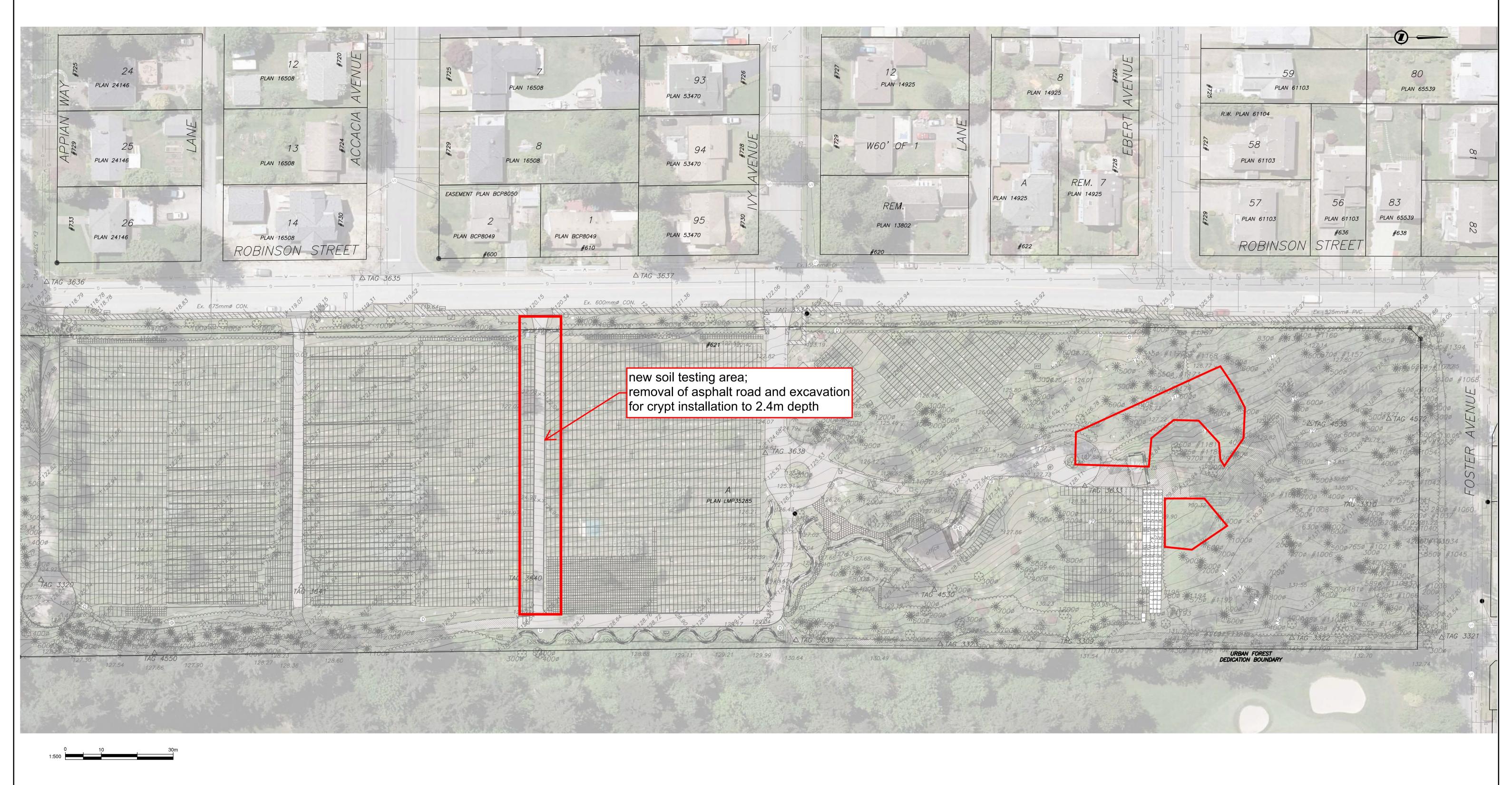
Appendix C – AGAT Certificates of Analysis (20V624985)

Appendix D – Results to CSR Comparison

References

Map showing sample locations at 621 Robinson Street, Coquitlam, BC, Google Earth, 2020.

APPENDIX A URBAN SYSTEMS SITE DRAWING



ISSUED FOR

2020-04-24 urbansystems.ca

50%

SUPPLEMENTARY LEGEND

House Number (Civic Address) Sanitary Cleanout

300 Vegetation Deciduous (diametre mm)

Vegetation Coniferous (diametre mm)

----- Embankment Bottoms

Fence/Building Post

Embankment Tops ---- Garden — — Ditch Top

_______Sidewalk Concrete

Brick

NOTES:

- 1. ELEVATIONS ARE DERIVED FROM G.P.S. OBSERVATIONS. DATUM IS [CGVD28 (GVRD 2005)]. TAG 3637 IS 121.154m, LOCATED AT SOUTHEAST CORNER OF #610 ROBINSON STREET
- 2. THIS PLAN SHOWS HORIZONTAL GROUND LEVEL MEASURED DISTANCES. PRIOR TO COMPUTATION OF NAD83 U.T.M. COORDINATES MULTIPLY BY THE COMBINED FACTOR 0.9995887. DATE OF SURVEY COMPLETION IS AUGUST 15, 2017.
- 3. THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE INDICATED IN THE PUBLIC ROAD ALLOWANCE ONLY AND ARE SHOWN APPROXIMATE. ONLY CITY UTILITIES ARE SHOWN. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. BC ONE-CALL PROVIDES UNDERGROUND UTILITY LOCATIONS (1–800–474–6886).
- 4. BASEMAP WAS IMPORTED FROM CITY OF COQUITLAM G.I.S. COORDINATE GEOMETRY. ACCURACY IS ESTIMATED AT ± 0.1 m.

Luge of puvernent		11)	urunt	-0-	Sumary Service	9
Watermain and valve	e — — w —	Wo	iter air valv	ve 🗅	Sanitary cleanout	
Drainage sewer, MH			iter blowoff	<u> </u>	Utility pole(joint pole)) =○=
Drainage ditch — —		Wo	iter service		Utility pole with light	⇔
Sanitary sewer, MH		Co	itch basin,	top inlet 🖾	Streetlight, davit	\bigcirc
Sanitary forcemain	SFM -	Co	ıtch basin,	side inlet ⊟	Streetlight, post top	0
Gasmain and valve	G	Co	ıtch basin,	round ⊘	Comb signal pole	<i>(TS</i>)—▽
Hydro duct, MH			ainage serv	vice —	Traffic signal pole	◆

→ Drainage cleanout ☐ Junction box

itary service———⑤	Hydro Guy Wire
itary cleanout \square	Hydro Kiosk
ty pole(joint pole) ←	Vegetation Conifer
ty pole with light 💬	Vegetation Deciduou
eetlight, davit	Vegetation Shrub
etlight, post top 🜼	Survey Traverse Hu
nb signal pole 🖾─▽	Survey Iron Pin
fin signal polo	Current Load Diva

Guy Wire Kiosk tion Conifer tion Deciduous tion Shrub	→ ⊞ ** ©					
Traverse Hub	\triangle		2020-04-24	BF	ISSUED FOR 50% DETAIL DESIGN	
Lead Plug		1	2020-04-24	BF	ISSUED FOR 25% DETAIL DESIGN	
Monument		No.	Date	Ву	Revisions	

-] [Design by M.KELDER	Date 2020-04-21
-	Drawn by M.KELDER	Date 2020-04-21
-	Checked by B.FAN	Date 2020-04-24
-][Approved by B.FAN	Date 2020-04-24



V3T 0K9

604-953-6500

Engineering & Public Works

3000 Guildford Way, Coquitlam, B.C. V3B 7N2

horiz. 1:500 ver Eng. Project No. 1150.0146.01

Project ROBINSON MEMORIAL PARK CEMETERY PHASE 1 IMPROVEMENTS EXISTING CONDITIONS / SITE PREPARATION PLAN

File: G02-1150014601-EXISTING

Telephone duct, MH Plot Date: April 24, 2020



APPENDIX B BOREHOLE LOGS

Project Name / Number: 10682 Robinson Cemetery

Client: Urban Systems Drilling Company: Omega

Date Drilled: 7/13/2020 10:20 **Drilling Method:** Solid Stem Auger

Site Location: North Wooded Area Logged By: AO Sheet: 1 of 1



SOIL PROFILE SAMPLE INFORMATION MW DETAILS Vapour Depth (m) Symbol Sample Depth **Description** Comments % ppm ft m ft m **-**0 Topsoil w/ some sand, (0-0.5)slight damp Brownish gray sand w/ Χ U some gravel, dry (1.0-1.5)(1.5-2.0)Gray sand, some brown, dry, some gravel Χ L EOH = 2.5m16. 19 19 Depth of Well (TOC):

Latitude: 49°15'21.48" N Longitude: 122°52'43.56" W Well Elevation (TOC):
Depth to Water Level (TOC): Ground Elevation:

Project Name / Number: 10682 Robinson Cemetery

Client: Urban Systems

Drilling Company: Omega
Drilling Method: Solid Ster 7/13/2020 9:30 Date Drilled: Solid Stem Auger

Site Location: North Wooded Area Logged By: AO Sheet: 1 of 1



	SOIL	PROFILE				SAMPLE IN	FORMATIC	DN	M	W DETAILS
	_		Œ,		<u>D</u>	Vapour	LEL			
Depth	Symbol	Description	Depth (m)	Туре	Sample ID	ppm 0 250 500	% 0 50 100	Comments	BH Diameter:	Depth
ft m		Ground Surface	0.00						Well Diameter:	ft m
00		Topsoil, damp	0.00	Χ	(0-0.5)				i	00
1 —				^	(0-0.5)					1
		Gray brown sand, some		Χ	U				lii	2
		orange tint, damp		^	O					-
3——1				Χ	(1.0-1.5)				li i	3—1
4				^	(1.0-1.3)					4
				Χ	(1.5-2.0)					5——
		Gray sand, damp			(1.0-2.0)					
6—2		S.S., Jana, Janp								6—2
7—				Χ	L					7—
8—		EOH = 2.5 m		^					li i	8—
9										9
									li i	´ _
10 3										103
11—									li i	11—
12—										12—
-									li i	4
13——4									H	13——4
14									li i	14
15—									H	15—
-										4
16—5										16—5
17—										17—
18——										18——
-										-
19—										19—
20—6		Major / Some / Trace Size / Consistency / Moisture / Color								206
		size / Consistency / Moisture / Color						Depth of Well (TOC):		
Latitude: 49°1 Depth to Water Le	5'20.57	™N Longitude:	122	°52'4	5.45" W			Well Elevation (TOC): Ground Elevation:		
Pebili io Maiel Fe	C 4 G1 (11	 						CIOUIU LIE VUIIOII.		

Project Name / Number: 10682 Robinson Cemetery

Client: Urban Systems Drilling Company: Omega

Date Drilled: 7/13/2020 9:07 **Drilling Method:** Solid Stem Auger

Site Location: North Wooded Area Logged By: AO Sheet: 1 of 1



SOIL PROFILE SAMPLE INFORMATION MW DETAILS Vapour Depth (m) Symbol Sample Depth **Description** Comments % ppm ft m m Topsoil, damp, trace (0-0.5)gravel Grayish brown, some Χ U gravel, slight damp (1.0-1.5) (1.5-2.0)Fine sand, gray, some brown staining Χ L EOH = 2.5m19 19 Depth of Well (TOC):

Latitude: 49°15'21.08" N Longitude: 122°52'45.79" W Well Elevation (TOC):
Depth to Water Level (TOC): Ground Elevation:

Project Name / Number: 10682 Robinson Cemetery

Client: Urban Systems Drilling Company: Omega

Date Drilled: 7/13/2020 8:42 **Drilling Method:** Solid Stem Auger

Site Location: North Wooded Area Logged By: AO Sheet: 1 of 1



SOIL PROFILE SAMPLE INFORMATION MW DETAILS Vapour Depth (m) Sample I Symbol Depth **Description** Comments % ppm ft m m **-**0 Topsoil, dark brown, (0-0.5)damp, some gravel Χ U Fine sand, grayish brown, some gravel (1.0-1.5) (1.5-2.0)Fine sand, gray, some brown staining Χ L EOH = 2.5m16. 19 19 Depth of Well (TOC):

Latitude: 49°15'21.59" N Longitude: 122°52'45.73" W Well Elevation (TOC):
Depth to Water Level (TOC): Ground Elevation:

Project Name / Number: 10682 Robinson Cemetery

Client: Urban Systems Drilling Company: Omega

Date Drilled: 7/13/2020 12:53 **Drilling Method:** Solid Stem Auger

Site Location: South Road Logged By: AO Sheet: 1 of 1



SOIL PROFILE SAMPLE INFORMATION MW DETAILS Vapour Depth (m) Symbol Sample Depth **Description** Comments ppm % ft m ft m **-**0 (0-0.5)Reddish brown loose damp sand w/ some gravel Χ U Compact damp grey silty sand w/ brown streaks (1.0-1.5)Wet dense silty clay w/ brown streaks Damp dense brownish grey (1.5-2.0)silty clay w/ reddish brown streaks Damp dark grey silty clay, high plastic Χ L $EOH = 2.5 \, m$ 19 19 Depth of Well (TOC):

Latitude: 49°15'14.58" N Longitude: 122°52'46.75" W Well Elevation (TOC):
Depth to Water Level (TOC): Ground Elevation:

Depth to Water Level (TOC):

Project Name / Number: 10682 Robinson Cemetery

Client: Urban Systems

Drilling Company: Omega
Drilling Method: Solid Ster 7/13/2020 12:15 Date Drilled: Solid Stem Auger

Site Location: South Road Logged By: AO Sheet: 1 of 1



SOIL PROFILE		SAMPLE INFORMATION						MW DETAILS	
Depth	Description	Depth (m)	Туре	Sample ID	Vapour ppm 0 250 500	% 0 50 100	Comments	BH Diameter:	Depth
ft m 0 - 0 1 - 2 2 - Re 3 - 1 4 - b 5 - Ve 6 - 2 7 - 3 11 - 12 - 13 - 4 14 - 15 - 16 - 5 17 - 18 - 19 - 2 20 - 6	Reddish brown loose damp sand w/ some gravel eddish brown grey clayey sand, damp, loose Compact grey w/ brown streak silty-clay, minor black smear ery dense grey clay w/ brown streaks Compact damp grey clayey silt EOH = 2.5 m	0.00	x	(0-0.5) U (1.0-1.5) L				Well Diameter:	ff m 0 - 0 1 - 2 3 - 1 4 - 2 7 - 2 8 - 9 10 - 3 11 - 3 11 - 3 12 - 3 13 - 4 14 - 5 16 - 5 17 - 18 - 5 17 - 18 - 5 17 - 6 20 - 6
	iize / Consistency / Moisture / Color	122°	°52'45	5.44" W			Depth of Well (TOC): Well Elevation (TOC): Ground Elevation:	1 1	

Ground Elevation:

Project Name / Number: 10682 Robinson Cemetery

Client: Urban Systems

Date Drilled: 7/13/2020 12:00 Drilling Company: Omega
Drilling Method: Solid Ster Solid Stem Auger

South Road Site Location: Logged By: AO Sheet: 1 of 1



SOIL PROFILE		SAMPLE INFORMATION						MW DETAILS		
Depth	Symbol	Description	Depth (m)	Iype	Sample ID	Vapour ppm 250 500	LEL % 0 50 100	Comments	BH Diameter:	Depth
## m 0	Symb	Brown clayey sand, damp w/ some cobble Brownish grey damp dense silty-clay, trace gravel EOH = 2.5 m	0.00	X X X X X X X X X X X X X X X X X X X	(0-0.5) U (1.0-1.5) (1.5-2.0)	ppm 0 250 500 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8	Comments	BH Diameter: Well Diameter:	######################################
20 — 6 Latitude: 49°1	15'14.57	Major / Some / Trace Size / Consistency / Moisture / Color "N Longitude:	122	2°52'4	4.45" W			Depth of Well (TOC): Well Elevation (TOC):		20——6

Ground Elevation: Depth to Water Level (TOC):

Project Name / Number : 10682 Robinson Cemetery

Client: Urban Systems Drilling Company: Omega

7/13/2020 10:40 Date Drilled: Drilling Method: Solid Stem Auger

South Road Site Location: Logged By: ΑO Sheet: 1 of 1



SOIL PROFILE SAMPLE INFORMATION MW DETAILS Vapour Depth (m) Symbol Sample Depth **Description Comments** ppm % ft m ft m **-**0 (0-0.5)Reddish brown sand some gray, w/ cobble and gravel, damp Χ Brownish gray sandy-clay, U dry. Some cobble/gravel (1.0-1.5) Dense dry grey clay w/ brown stains (1.5-2.0)Grey brown clayey silt w/ cobble/gravel Χ L EOH = 2.3 mRefusal due to large boulder 5 19 19 Depth of Well (TOC):

49°15'14.55" N **Longitude:** Latitude: 122°52'43.43" W Depth to Water Level (TOC): **Ground Elevation:**

Well Elevation (TOC):